

بسم الله الرحمن الرحيم

«قال يا قوم أرايتم إن كنت على بينة من ربي ورزقني منه رزقا حسنا
وما أريد أن أخالفكم إلى ما أنهاكم عنه إن أريد إلا الإصلاح ما استطعت
وما توفيقي إلا بالله عليه توكلت وإليه أنيب»



صدق الله العظيم

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تصميم اجزاء ماكينات (١) – همك ٤١١

Design of Machine Elements (1) – ME 411

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1st Term AY 1441-1442/2020-2021



الاسبوع الرابع – المحاضرة الرابعة



التعليم عن بعد

الانحناء في الكمرات



Distant Learning

Beam Deflection

Design Concepts

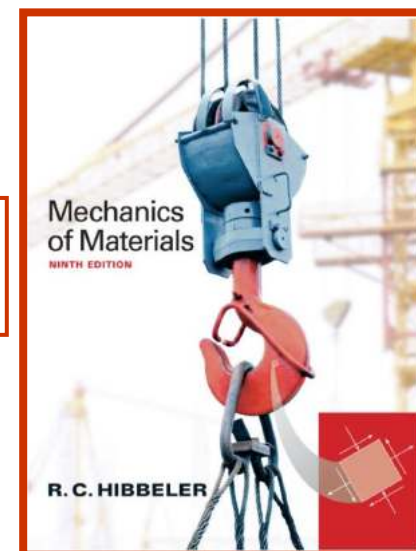
- ✓ Design based on Stresses
- ✓ Design based on Stiffness

Chapter 12



If the curvature of this pole is measured, it is then possible to determine the bending stress developed within it.

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Design Concepts

- ✓ Design based on Stresses
- ✓ Design based on Stiffness

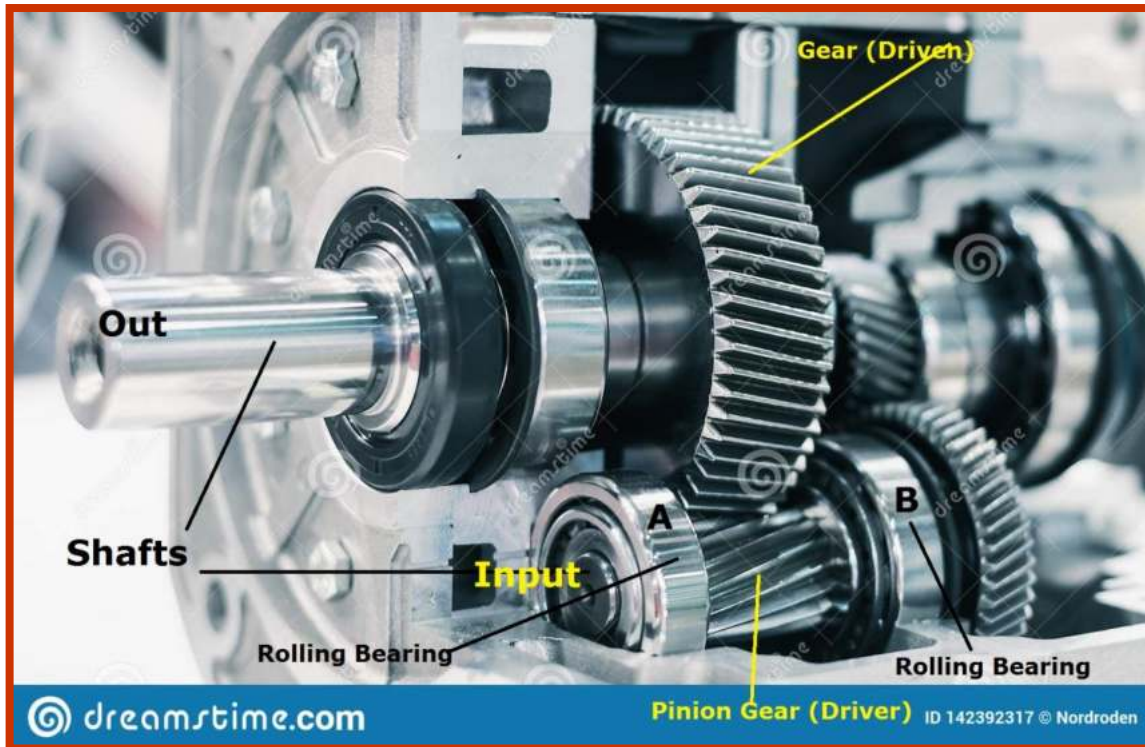
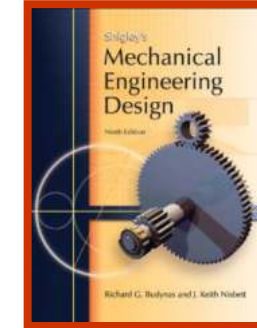


Table 7-2

Typical Maximum
Ranges for Slopes and
Transverse Deflections

Slopes	
Tapered roller	0.0005–0.0012 rad
Cylindrical roller	0.0008–0.0012 rad
Deep-groove ball	0.001–0.003 rad
Spherical ball	0.026–0.052 rad
Self-align ball	0.026–0.052 rad
Uncrowned spur gear	< 0.0005 rad
Transverse Deflections	
Spur gears with $P < 10$ teeth/in	0.010 in
Spur gears with $11 < P < 19$	0.005 in
Spur gears with $20 < P < 50$	0.003 in

After: <https://www.dreamstime.com/mechanical-gearbox-cross-section-helical-gear-close-up-image142392317>

Slope and Deflection vs. $M_{(x)}$

Elastic Curve

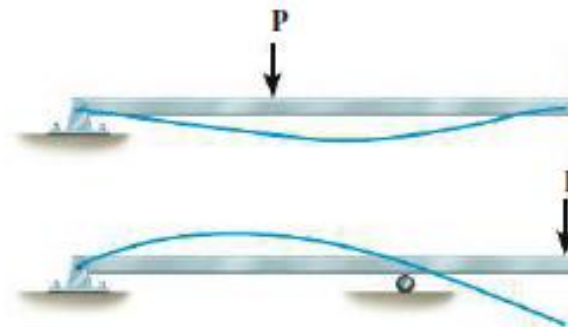
where ρ is the radius of curvature. From studies in mathematics we also learn that the curvature of a plane curve is given by the equation

$$\frac{1}{\rho} = \frac{d^2y/dx^2}{[1 + (dy/dx)^2]^{3/2}}$$

where the interpretation here is that y is the lateral deflection of the centroidal axis of the beam at any point x along its length. The slope of the beam at any point x is

$$\frac{1}{\rho} = \frac{M}{EI}$$

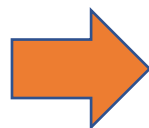
$$\theta = \frac{dy}{dx}$$



For many problems in bending, the slope is very small,

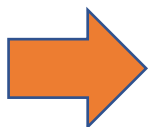
$$\frac{M}{EI} = \frac{d^2y}{dx^2}$$

$$EI \frac{dy}{dx} = \int M dx$$

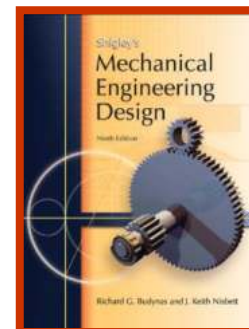


Slope = θ

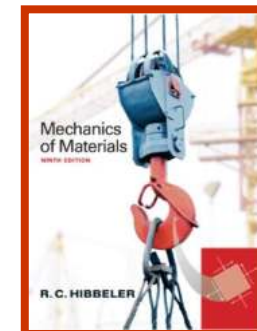
$$EI y = \iint M dx$$



Deflection = $y = \delta$



Ch.4 p. 147



Ch. 12 p.573

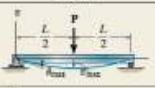
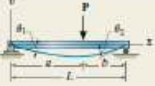
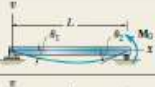
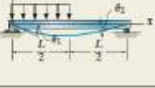
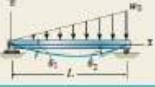
Deflection and Slope Correlations in Different Types of Supported and Loaded Beams

APPENDIX

C

Slopes and Deflections of Beams

Simply Supported Beam Slopes and Deflections

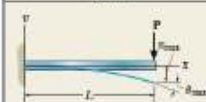
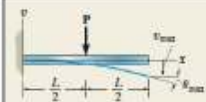
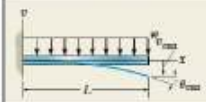
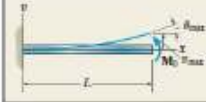
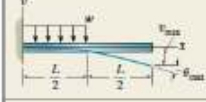
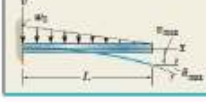
Beam	Slope	Deflection	Elastic Curve
	$\theta_{max} = \frac{-PL^2}{16EI}$	$v_{max} = \frac{-PL^3}{48EI}$	$v = \frac{-Px}{48EI} (3L^2 - 4x^2)$ $0 \leq x \leq L/2$
	$\theta_1 = \frac{-Pab(L+b)}{6EIL}$ $\theta_2 = \frac{Pab(L+a)}{6EIL}$	$v _{x=a} = \frac{-Pbx}{6EIL} (L^2 - b^2 - a^2)$	$v = \frac{-Pbx}{6EIL} (L^2 - b^2 - x^2)$ $0 \leq x \leq a$
	$\theta_1 = \frac{-M_0L}{6EI}$ $\theta_2 = \frac{M_0L}{3EI}$	$v_{max} = \frac{-M_0L^2}{9\sqrt{3}EI}$ at $x = 0.5774L$	$v = \frac{-M_0x}{6EIL} (L^2 - x^2)$
	$\theta_{max} = \frac{-wL^3}{24EI}$	$v_{max} = \frac{-5wL^4}{384EI}$	$v = \frac{-wx}{24EI} (x^3 - 2Lx^2 + L^3)$
	$\theta_1 = \frac{-3w_0L^3}{128EI}$ $\theta_2 = \frac{7w_0L^3}{384EI}$	$v _{x=L/2} = \frac{-5w_0L^4}{768EI}$ $v_{max} = \frac{-0.006563}{EI} w_0L^4$ at $x = 0.4598L$	$v = \frac{-wx}{384EI} (16x^3 - 24Lx^2 + 9L^3)$ $0 \leq x \leq L/2$ $v = \frac{-wL}{384EI} (8x^3 - 24Lx^2 + 17L^2x - L^3)$ $L/2 \leq x < L$
	$\theta_1 = \frac{-7w_0L^3}{360EI}$ $\theta_2 = \frac{w_0L^3}{45EI}$	$v_{max} = \frac{-0.00652}{EI} w_0L^4$ at $x = 0.5193L$	$v = \frac{-w_0x}{360EI} (3x^4 - 10L^2x^2 + 7L^4)$

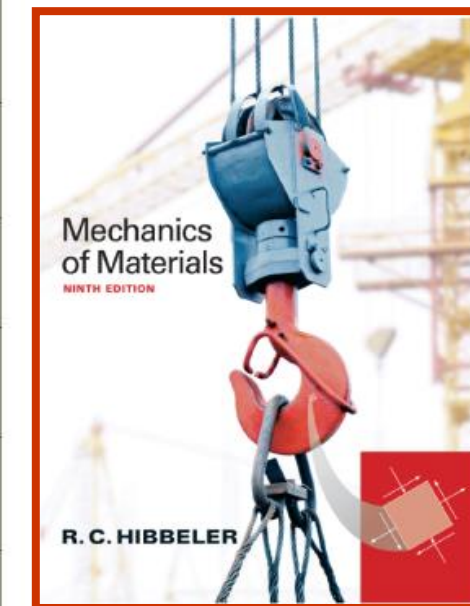
812

CANTILEVERED BEAM SLOPES AND DEFLECTIONS

813

Cantilevered Beam Slopes and Deflections

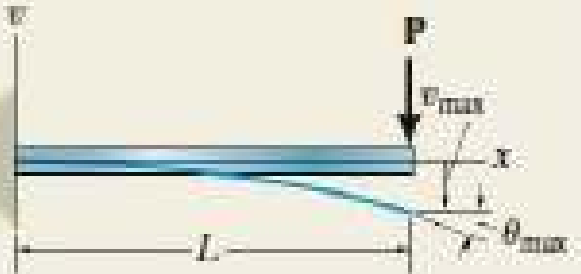
Beam	Slope	Deflection	Elastic Curve
	$\theta_{max} = \frac{-PL^2}{2EI}$	$v_{max} = \frac{-PL^3}{3EI}$	$v = \frac{-Px^2}{6EI} (3L - x)$
	$\theta_{max} = \frac{-PL^2}{8EI}$	$v_{max} = \frac{-5PL^3}{48EI}$	$v = \frac{-Px^2}{12EI} (3L - 2x)$ $0 \leq x \leq L/2$ $v = \frac{-Px^2}{48EI} (6x - L)$ $L/2 \leq x \leq L$
	$\theta_{max} = \frac{-wL^3}{6EI}$	$v_{max} = \frac{-wL^4}{8EI}$	$v = \frac{-wx^2}{24EI} (x^2 - 4Lx + 6L^2)$
	$\theta_{max} = \frac{M_0L}{EI}$	$v_{max} = \frac{M_0L^2}{2EI}$	$v = \frac{M_0x^2}{2EI}$
	$\theta_{max} = \frac{-wL^3}{48EI}$	$v_{max} = \frac{-7wL^4}{384EI}$	$v = \frac{-wx^2}{24EI} (x^2 - 2Lx + \frac{2}{3}L^2)$ $0 \leq x \leq L/2$ $v = \frac{-wL^2}{384EI} (8x - L)$ $L/2 \leq x \leq L$
	$\theta_{max} = \frac{-w_0L^3}{24EI}$	$v_{max} = \frac{-w_0L^4}{30EI}$	$v = \frac{-w_0x^2}{120EI} (10L^3 - 10L^2x + 5Lx^2 - x^3)$



2

Beam Deflection Derivation

Case No. (1) Tabulated

Cantilevered Beam Slopes and Deflections			
Beam	Slope	Deflection	Elastic Curve
	$\theta_{max} = \frac{-PL^2}{2EI}$	$v_{max} = \frac{-PL^3}{3EI}$	$v = \frac{-Px^2}{6EI} (3L - x)$

$$EI(\theta) = \frac{Px^2}{2} - PLx$$

$$EI(\delta) = \frac{Px^3}{6} - \frac{PLx^2}{2}$$

δ
Delta

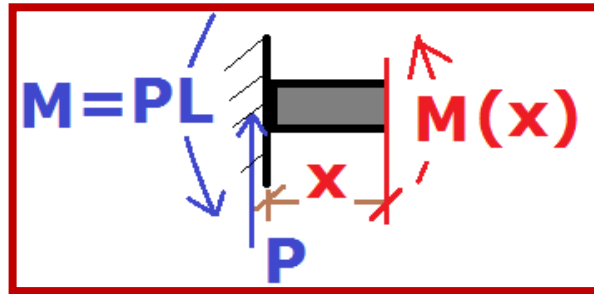
Beam Deflection Derivation

Case No. (1)

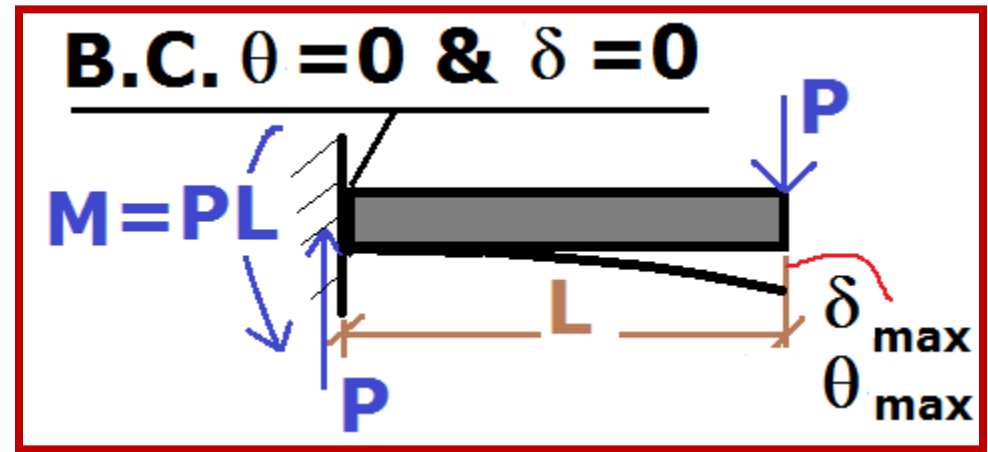
Derived by Integration Method

$$EI(\theta) = \int M(x) dx$$

$$EI(\delta) = \iint M(x) dx$$



$$M(x) = Px - PL$$



$$EI(\theta) = \int (Px - PL) dx = \frac{Px^2}{2} - PLx + C_1 \Rightarrow @ x=0; \theta=0 \text{ gives } C_1=0$$

$$EI(\delta) = \iint M(x) dx = \frac{Px^3}{6} - \frac{PLx^2}{2} - C_1x + C_2 \Rightarrow @ x=0; \delta=0 \text{ gives } C_2=0$$

Beam Design on Both Stresses and Deflection

Solved Problem (Practice No.1)

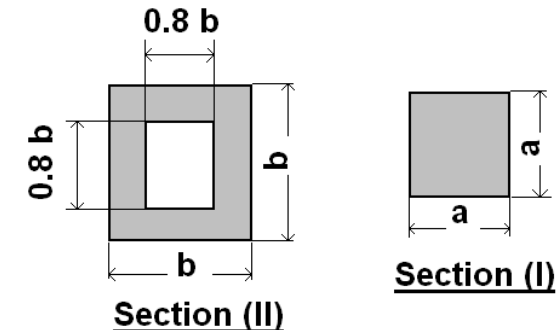
1. Design standard two cross-sections: Solid square (section I), and Box square (Section II) (Use static flexural stress formula only).
2. Calculate the maximum deflection in (mm) and slope in (rad) of the loaded beam for each cross-section and identify its position.
3. Comment on the previous results and state one conclusion.

Given:

Young's modulus = 210 GPa, yield strength = 390 MPa
and Design factor = 3, $P = 10$ kN, $L = 1$ m



Cantilevered Beam



Two cross-sections



Beam Design on Both Stresses and Deflection

Solved Problem (Practice No.1)



$M_{(max)} = PL = 10 \times 1 = 10 \text{ (kN.m)}$ at the support as $[M_{(max)} / Z] = [\sigma_y / n_d]$

$Z = (10 \times 1000 \times 1000) \text{ N.mm} / 130 \text{ (N/mm}^2\text{)} = 76.9231 \times 1000 \text{ (mm}^3\text{)}$

$a = 42.53 \text{ mm}$, for standard **use $a_{st.} = 45 \text{ mm}$**

$b = 98.16 \text{ mm}$, for standard **use $b_{st.} = 100 \text{ mm}$**

 $I \text{ (moment of Inertia) for solid square} = a_{st}^4 / 12 = 341718.75 \text{ mm}^4$

$I \text{ (moment of Inertia) for Box square} = (b_{st}^4 - (0.8b)^4) / 12 = 4920000 \text{ mm}^4$

Maximum Deflection at free end = $PL^3 / 3EI =$

$\{[(10000(1000)^3 \text{ N.mm}^3)] / [3(210 \times 1000 \text{ N/mm}^2)(I)]\} = \{15873015.87 / (I)\} \text{ (mm)}$

Maximum Slope at free end = $PL^2 / 2EI =$

$\{[(10000(1000)^2 \text{ N.mm}^2)] / [2(210 \times 1000 \text{ N/mm}^2)(I)]\} = \{500000 / 21(I)\} \text{ (rad.)}$



Beam Design on Both Stresses and Deflection **Solved Problem (Practice No.1)**



For **solid square cross-section** (Maximum Deflection) = **47 mm**

For **solid square cross-section** (Maximum Slope) = **0.0697 rad. = 4 degrees**

For **Box square cross-section** (Maximum Deflection) = **3.2 mm**

For **Box square cross-section** (Maximum Slope) = **0.00048 rad. = 0.3 degree**

Comments

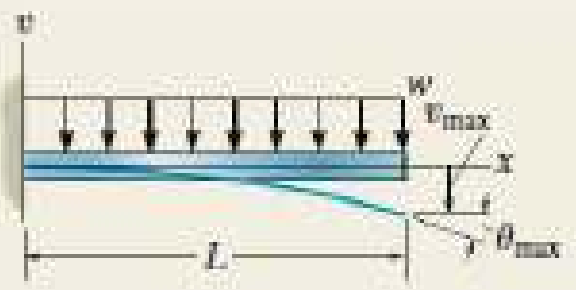
1- Deflection and Slope of the loaded beam showed lower values by using box structure cross section instead of solid square cross section.

2- Beams with box structure cross-section not only lighter in weight but also behave more rigid (stiff) in its elastic curve (deflection and slope) than solid square cross-section

Conclusion: Cross-section profile and orientation is very important parameter in designing of beams; not only to save weight (money) but also to make firm, stiff and rigid beam in its applications.

Beam Deflection Derivation

Case No. (2) Tabulated

Cantilevered Beam Slopes and Deflections			
Beam	Slope	Deflection	Elastic Curve
	$\theta_{\max} = \frac{-wL^3}{6EI}$	$v_{\max} = \frac{-wL^4}{8EI}$	$v = \frac{-wx^2}{24EI} (x^2 - 4Lx + 6L^2)$

Δ
Delta



Beam Deflection Derivation



Case No. (2)

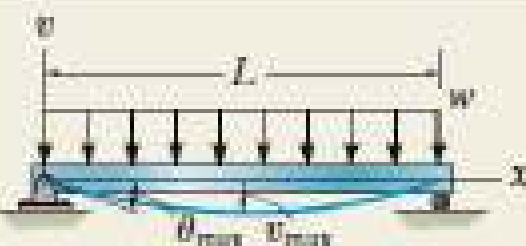
Derived by Integration Method

**Two Pones points for all students in tomorrow
practice (Competition)**

**And in case of no student contribution
T.B.S and I in Practical Lecture No. 4 (Tomorrow)**

Beam Deflection Derivation

Case No. (3) Tabulated

Simply Supported Beam Slopes and Deflections			
Beam	Slope	Deflection	Elastic Curve
	$\theta_{max} = \frac{-wL^3}{24EI}$	$v_{max} = \frac{-5wL^4}{384EI}$	$v = \frac{-wx}{24EI} (x^3 - 2Lx^2 + L^3)$

δ
Delta

Beam Deflection Derivation

Case No. (3)

Derived by Integration Method

Deflection and Stiffness | 151

EXAMPLE 4-1 For the beam in Fig. 4-2, the bending moment equation, for $0 \leq x \leq l$, is

$$M = \frac{wl}{2}x - \frac{w}{2}x^2$$

Using Eq. (4-12), determine the equations for the slope and deflection of the beam, the slopes at the ends, and the maximum deflection.

Solution Integrating Eq. (4-12) as an indefinite integral we have

$$EI \frac{dy}{dx} = \int M dx = \frac{wl}{4}x^2 - \frac{w}{6}x^3 + C_1 \quad (1)$$

where C_1 is a constant of integration that is evaluated from geometric boundary conditions. We could impose that the slope is zero at the midspan of the beam, since the beam and

loading are symmetric relative to the midspan. However, we will use the given boundary conditions of the problem and verify that the slope is zero at the midspan. Integrating Eq. (1) gives

$$EIy = \iint M dx = \frac{wl}{12}x^3 - \frac{w}{24}x^4 + C_1x + C_2 \quad (2)$$

The boundary conditions for the simply supported beam are $y = 0$ at $x = 0$ and l . Applying the first condition, $y = 0$ at $x = 0$, to Eq. (2) results in $C_2 = 0$. Applying the second condition to Eq. (2) with $C_2 = 0$,

$$EIy(l) = \frac{wl}{12}l^3 - \frac{w}{24}l^4 + C_1l = 0$$

Solving for C_1 yields $C_1 = -wl^3/24$. Substituting the constants back into Eqs. (1) and (2) and solving for the deflection and slope results in

$$y = \frac{wx}{24EI}(2lx^2 - x^3 - l^3) \quad (3)$$

$$\theta = \frac{dy}{dx} = \frac{w}{24EI}(6lx^2 - 4x^3 - l^3) \quad (4)$$

Comparing Eq. (3) with that given in Table A-9, beam 7, we see complete agreement. For the slope at the left end, substituting $x = 0$ into Eq. (4) yields

$$\theta|_{x=0} = -\frac{wl^3}{24EI}$$

and at $x = l$,

$$\theta|_{x=l} = \frac{wl^3}{24EI}$$

At the midspan, substituting $x = l/2$ gives $dy/dx = 0$, as earlier suspected.

The maximum deflection occurs where $dy/dx = 0$. Substituting $x = l/2$ into Eq. (3) yields

$$y_{\max} = -\frac{5wl^4}{384EI}$$



الاسبوع الخامس – المحاضرة الخامسة



التعليم عن بعد

تصميم الكمرات على الاجهاد والانحناء – مسائل محلولة



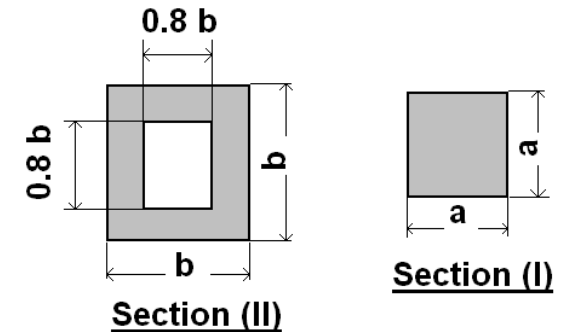
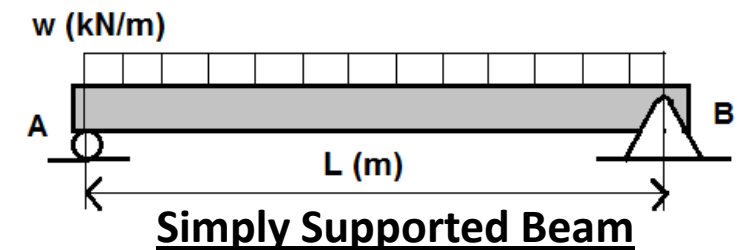
Distant Learning

Beam Design on both stresses and Deflection Solved Problems

Beam Design on Both Stresses and Deflection

Solved Problem (Drill No. 1)

1. Deduce the deflection and slope equations for the shown simply supported beam shown in Figure (1).
2. Design standard two cross-sections: Solid square (section I), and Box square (Section II) (Use static flexural stress formula only).
3. Get the factor of safety for each standard cross-section to the corrected nearest 5 mm of each side length.
4. Calculate the weight of the beam for each cross-section in (kg) and identify the lighter beam weight?
5. Calculate the maximum deflection in (mm) and slope in (rad) of the loaded beam for each cross-section and identify its position.
6. Comment on the previous results (parts 4 and 5) and state one conclusion.



Two cross-sections

Assume the loaded beam has the following data

Young's modulus = 210 GPa, yield strength = 390 MPa and Design factor = 3

$w = 4 \text{ kN/m}$, $L = 1 \text{ m}$ and material's density = 7.8 g/cm^3

بسم الله الرحمن الرحيم

«يوم تجد كل نفس ما عملت من خير محضراً وما
عملت من سوء تورد له ان بينها وبينه امداً
بجهداً» ويذكركم الله نفسه والله رءوف بالعباد» صدق الله العظيم

٣٠ آل عمران

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